

# Design of Buried Thermoplastics Pipes

The background features a cross-sectional diagram of a pipe buried in the ground. The pipe is shown as a horizontal cylinder with a grid-like texture, representing its internal structure. The ground is depicted in shades of brown and orange. A large, semi-transparent yellow arrow points from the left towards the right, passing through the pipe. The overall design is clean and technical, with a focus on the pipe's structure and its placement in the ground.

Results of a European research project  
by APME & TEPPFA

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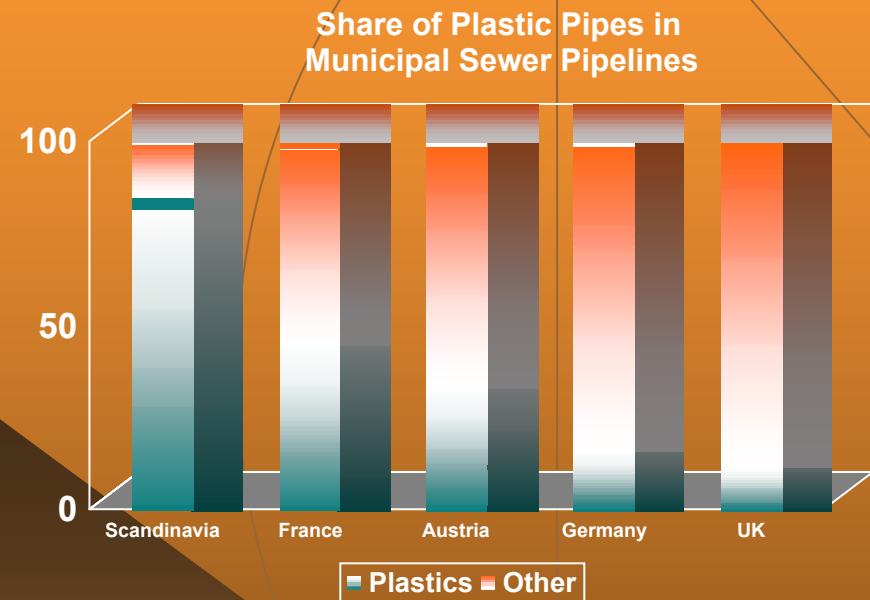


# Organisations supporting the project

- ◆ **TEPPFA**  
The European Plastics Pipe and Fitting Association
- ◆ **APME**  
Association of Plastics Manufacturers in Europe

# Current situation

- ◆ Rigid materials still dominate on many European markets.
- ◆ Prevailing design practices often tailored for rigid pipes.
- ◆ Flexibility considered as a weakness.
- ◆ Designers not always familiar with the behavior of plastic pipes when buried underground.



# Misconceptions about plastics pipes

- ◆ Deflection increases with installation depth and with traffic load.
  - ◆ Pipe ring stiffness is the governing factor determining the performance.
  - ◆ Pipe loses stiffness with time, the load bearing capacity reduces.
  - ◆ To predict the structural performance an extensive design method is needed.
  - ◆ Flexible behaviour is a disadvantage.
  - ◆ Deflected pipe loses its discharge capacity and tightness.
- TEPPFA and APME started an extensive research project to address these arguments.



# Try doing this with plastics



Source:  
*American Concrete Association*



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# Objectives of the project

- ◆ Show the relative importance of the parameters.
- ◆ Prove flexibility to be a strength instead of a disadvantage.
- ◆ Develop a design approach in balance with achievable installation quality and actual behaviour.
- ◆ Contribute to the development of the European standards with real field trials / test results.
- ◆ Provide material to communicate the project results to the marketplace.



# Project Group

- ◆ Frans Alferink      Wavin M&T      Project manager (NL)
- ◆ Lars-Eric Janson      SWECO      Supervisor (S)
- ◆ Jonathan Olliff      Montgomery /  
Watson      Supervisor (UK)





# Steering Committee

Name	Company / Association		
Ingemar Björklund	KWH Pipe / NPG	(S)	Chairman
Helmut Leitner	Solvay / APME	(B)	
Tiem Meijering	Polva-Pipelife / FKS (NL)		
Michael Giay	REHAU / ON	(A)	
Dieter Scharwächter	Uponor / KRV	(D)	
Jacques Nury	Alphacan / STRPE-PVC	(F)	
Constantino Gonzalez	ITEPE / ASETUB	(E)	
Alan Headford	Durapipe-S&LP / BPF	(UK)	
Jukka Kallioinen	Uponor	(D)	
Loek Wubbolt	Omnoplast	(NL)	
Trefor Jones	Wavin	(UK)	
Frans Alferink	Wavin M&T	(NL)	Secretary



# Project set-up

Started in July 1996, Costs : Euro 450.000,=

- ◆ Full scale field trials with different materials, stiffnesses, soils and installation conditions carried out in Haarle and Wons (NL), involving:
  - ◆ Traffic load simulations
  - ◆ Depth variations
  - ◆ Internal pressure
  - ◆ Time effect
- ◆ Supporting laboratory tests.
- ◆ Design exercises together with leading European experts to compare existing calculation methods with results from field measurements.
- ◆ Evaluation with European design experts in a workshop.



# European experts involved

Expert	Design Method	Country
	<b><i>EN 1295</i></b>	
Günther Leonhardt	ATV A 127	(Germany)
Marcel Gerbault	Fascicule 70	(France)
Walther Netzer	ÖNORM B 5012	(Austria)
Lars-Eric Janson	VAV P70	(Sweden)
Jonathan Olliff	PSSM	(United Kingdom)
	<b><i>Others</i></b>	
Hubert Schneider	GRP-draft	(Germany)
Frans Alferink	CaVis	(The Netherlands)
Tiem Meijering	Bossen	(The Netherlands)



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# Approach with European design experts

Step	Activity
1	Consultation with experts regarding field trials.
2	European experts calculating the pipe deflections by using the different methods.
3	Establishing test fields and carrying out extensive measurements.
4	Continuing the field measurements at defined times.
5	Evaluation of all results in a two day workshop (December 1997).



# The field trials : Installed pipes

Material	Stiffness [kN/m <sup>2</sup> ]	Cover [m]	Installed length [m]
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## *Silty sand*, November 1996

PVC	2 and 4	1.15	120
		1.85	60
PE	5	1.15	45
Steel	4	1.85	20

## *Silty clay*, August 1997

PE	5	1.15	60
		3.0	60



# Documented test data

## ◆ Soil

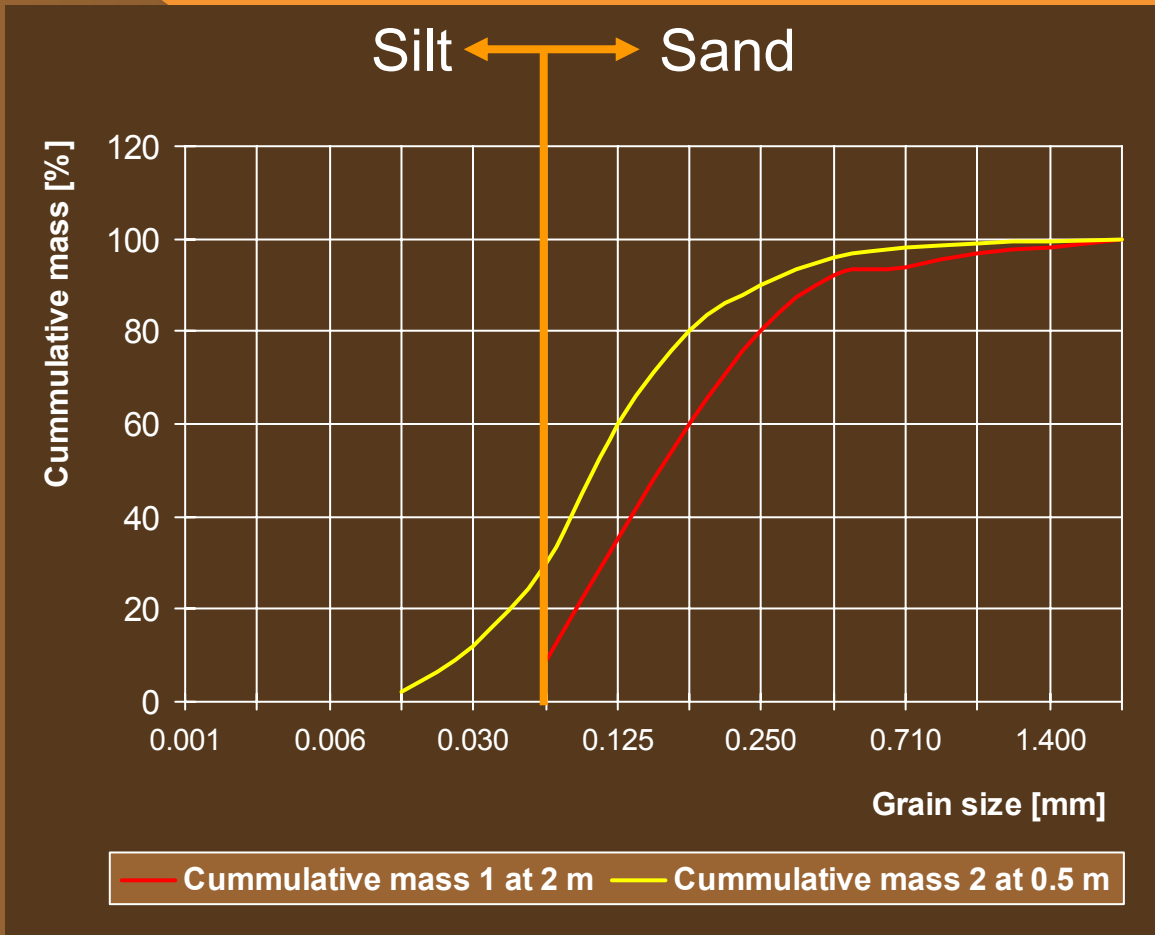
- ◆ Grain size distribution
- ◆ Grain shape
- ◆ Proctor density
- ◆ Menard test
- ◆ Cone penetration test
- ◆ Tri-axial test (clay)
- ◆ Cone-pressiometer test
- ◆ Impact cone test
- ◆ Oedometer test

## ◆ Pipe

- ◆ Dimensions
- ◆ Stiffness
- ◆ Creep ratio
- ◆ Deflections
  - ◆ time dependency
  - ◆ under internal pressure
  - ◆ under traffic load
  - ◆ under ground water
- ◆ Strain under deformation



# Natural variations in soil



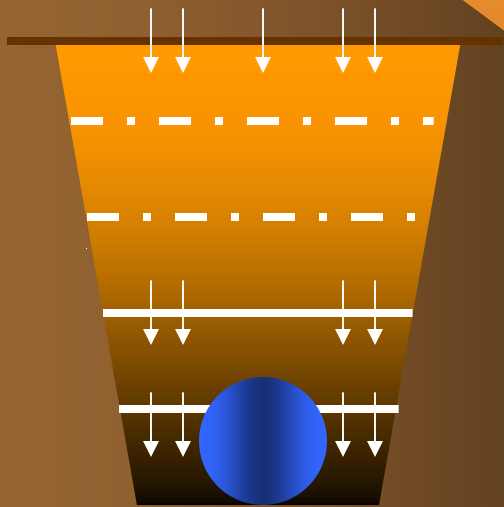
Grain size distributions of sand taken at two different depths



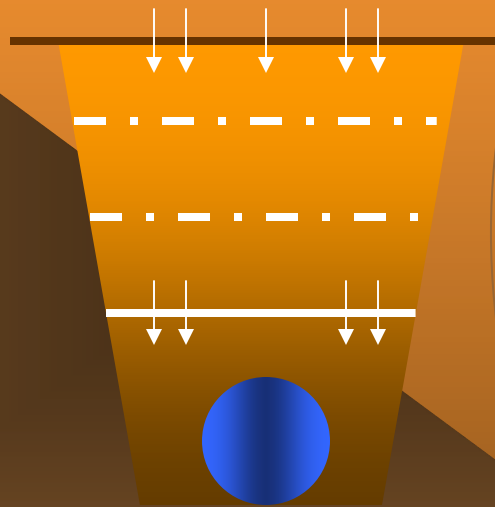


# Installation practices used in the project

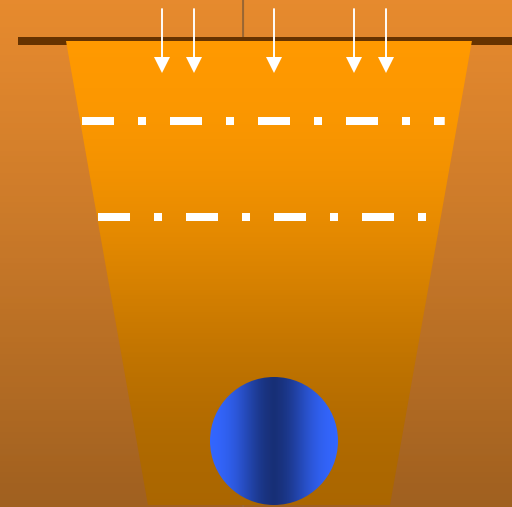
Well



Moderate

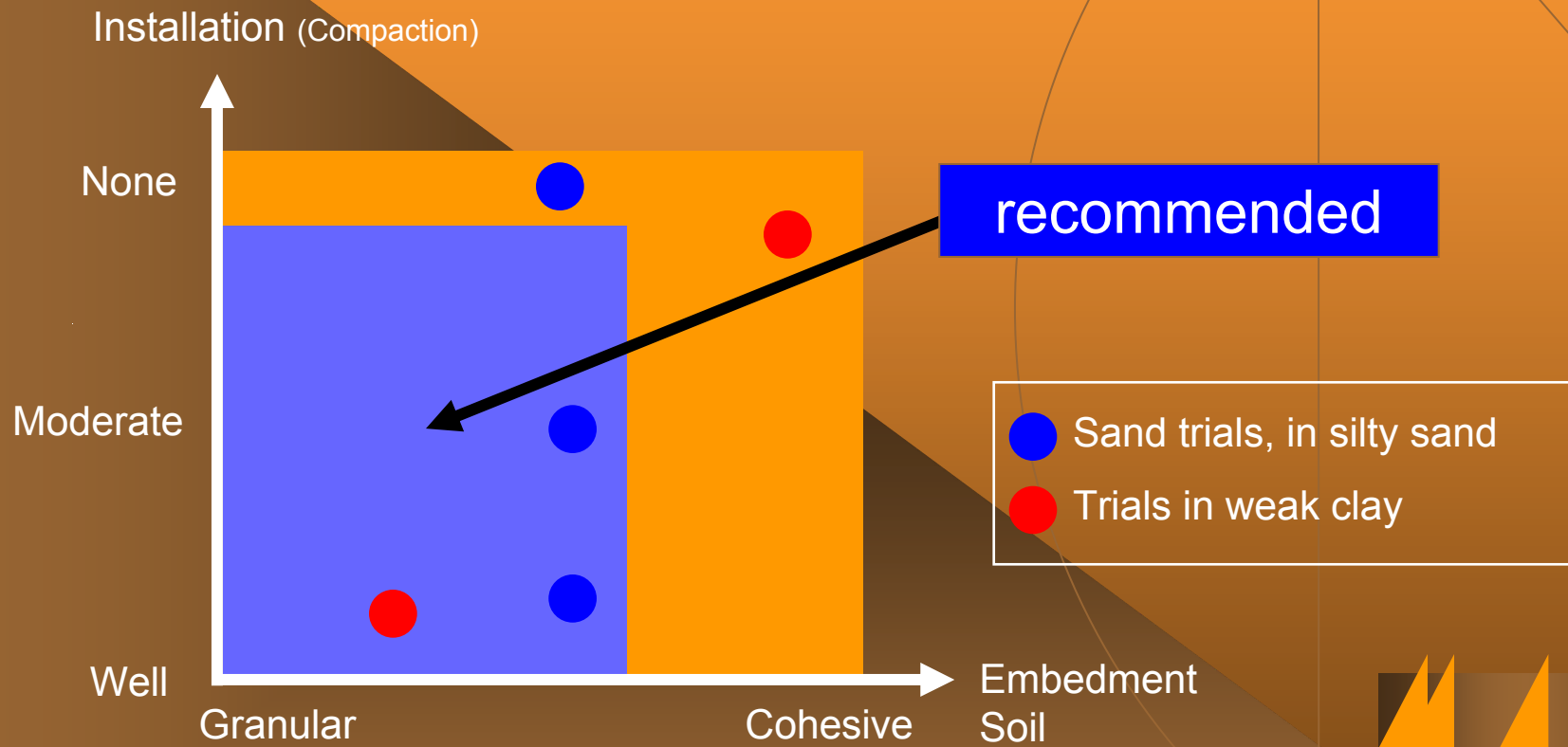


None



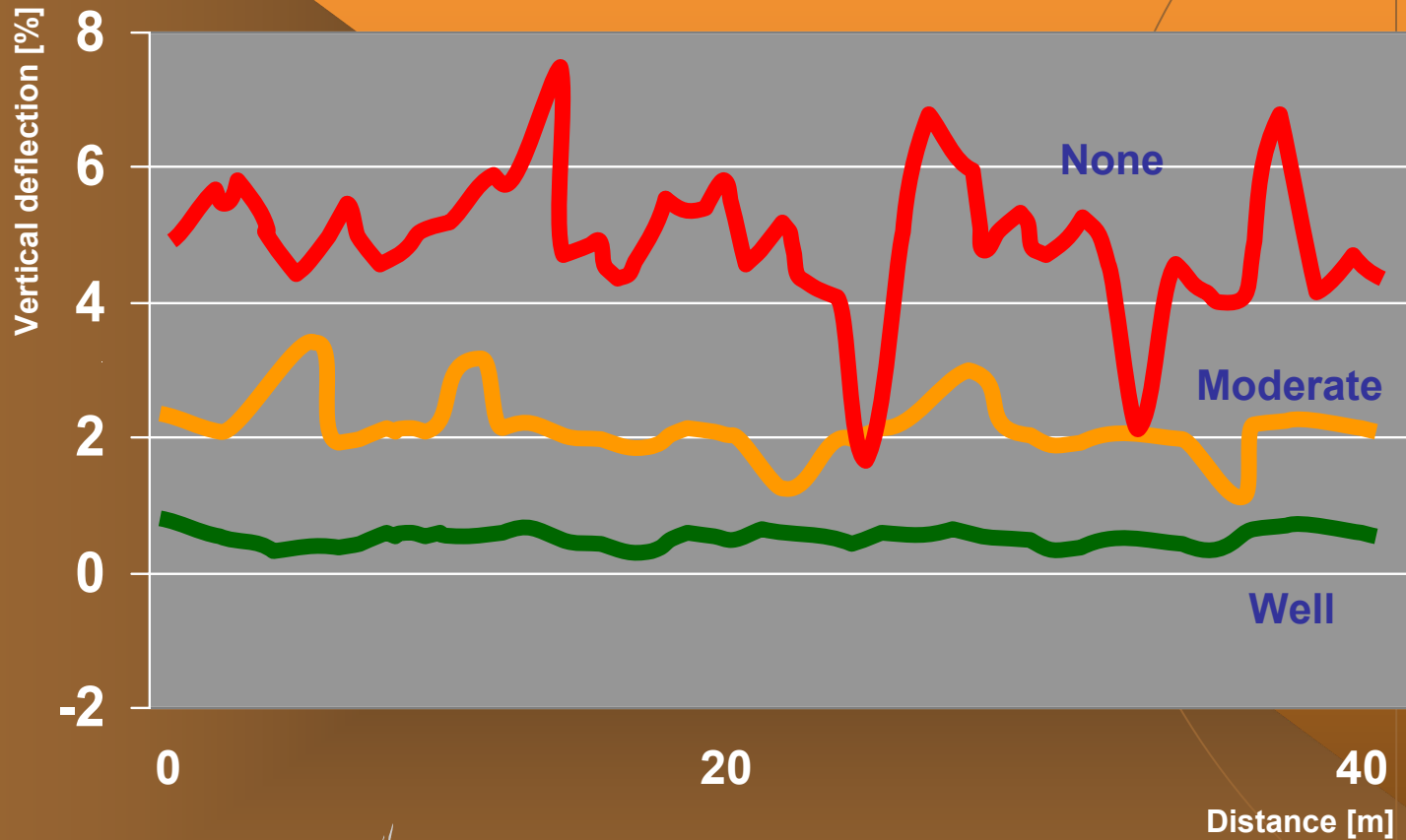
# Position of trials

## Position of trials in generalised application window



# Pipe deflection

Measured deflections for different types of installation



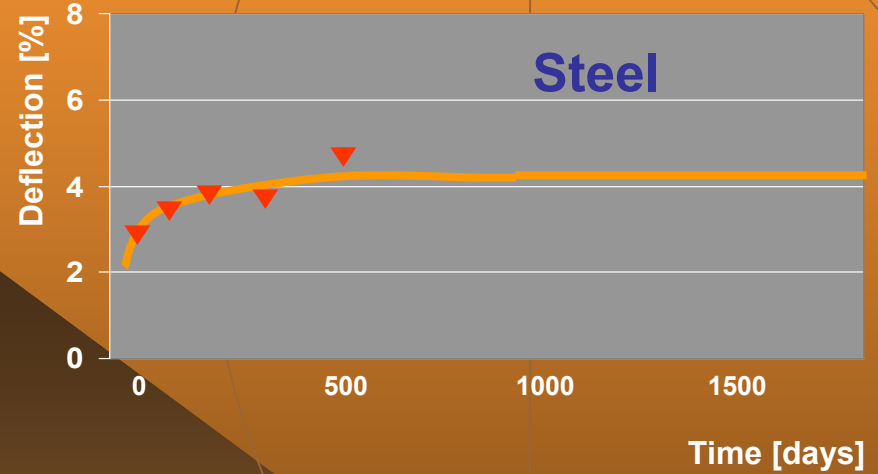
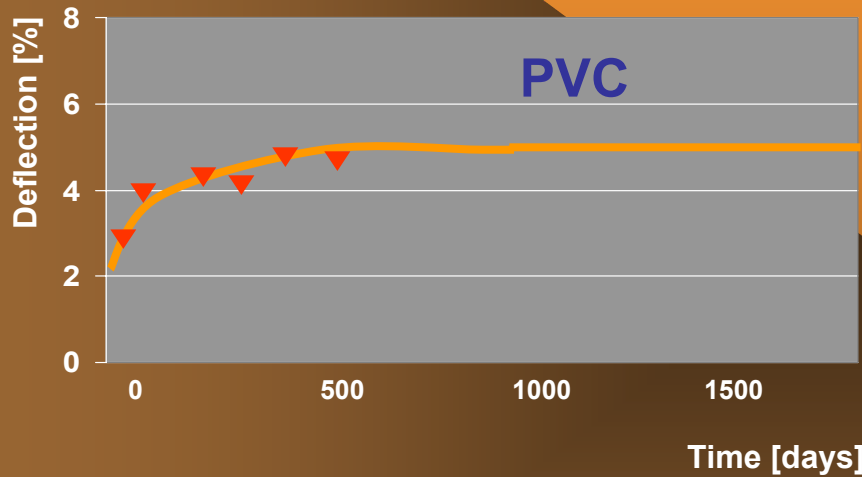
# Findings from workshop discussions

- ◆ "Installation of pipeline systems varies from meter to meter depending on many aspects such as workmanship, native soil variations, weather conditions and logistics in the field."
- ◆ "Consequently, the installation variability results in variations in ring deflection along the pipeline for flexible pipes and in variations in bending moments along the pipeline for rigid and semi-rigid pipes."



# No difference between PVC / Steel

Time dependency of the deflection

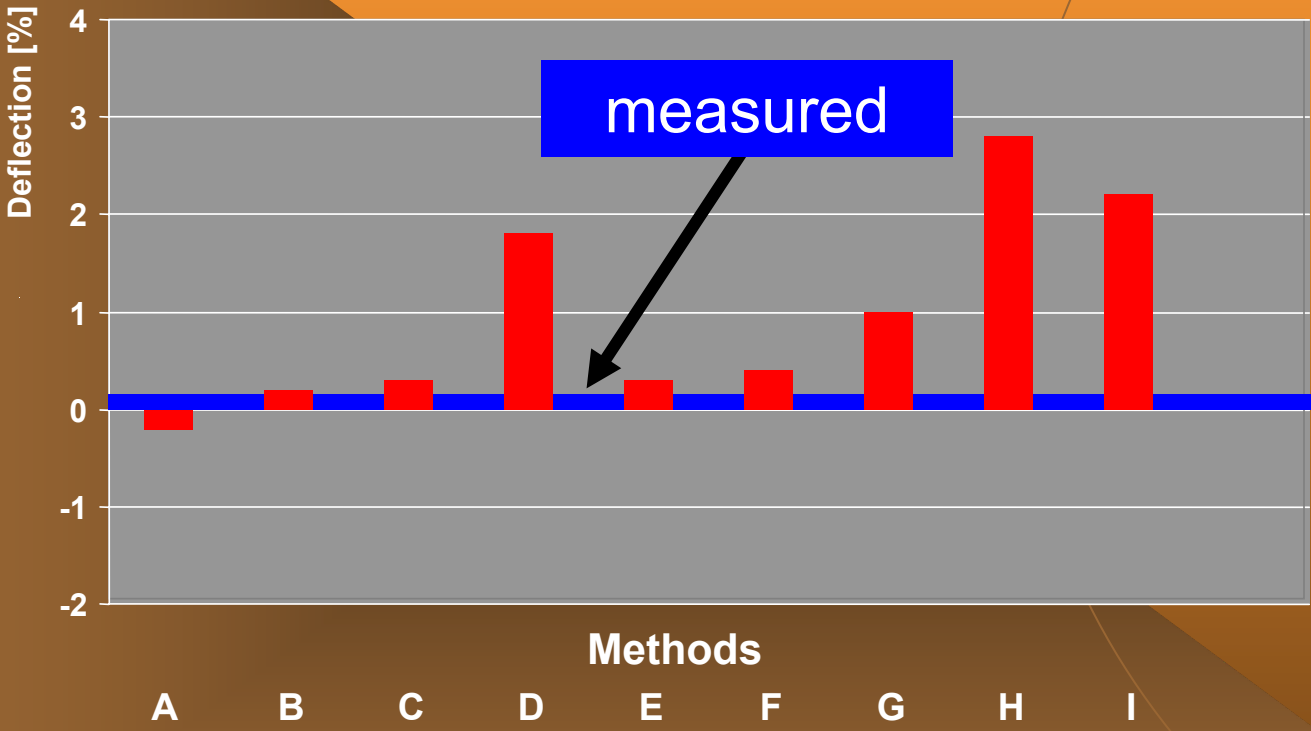


Measured



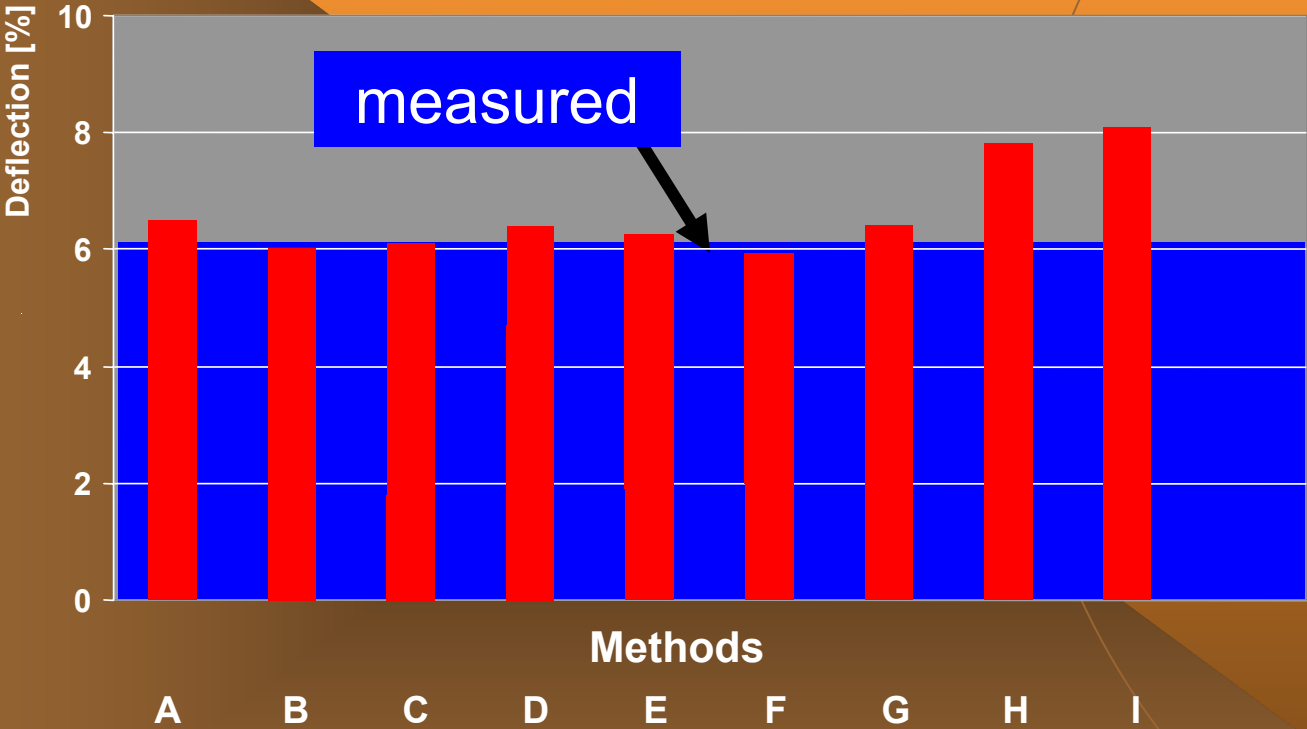
# Calculated and measured deflections

Granular soil, good installation



# Calculated and measured deflections

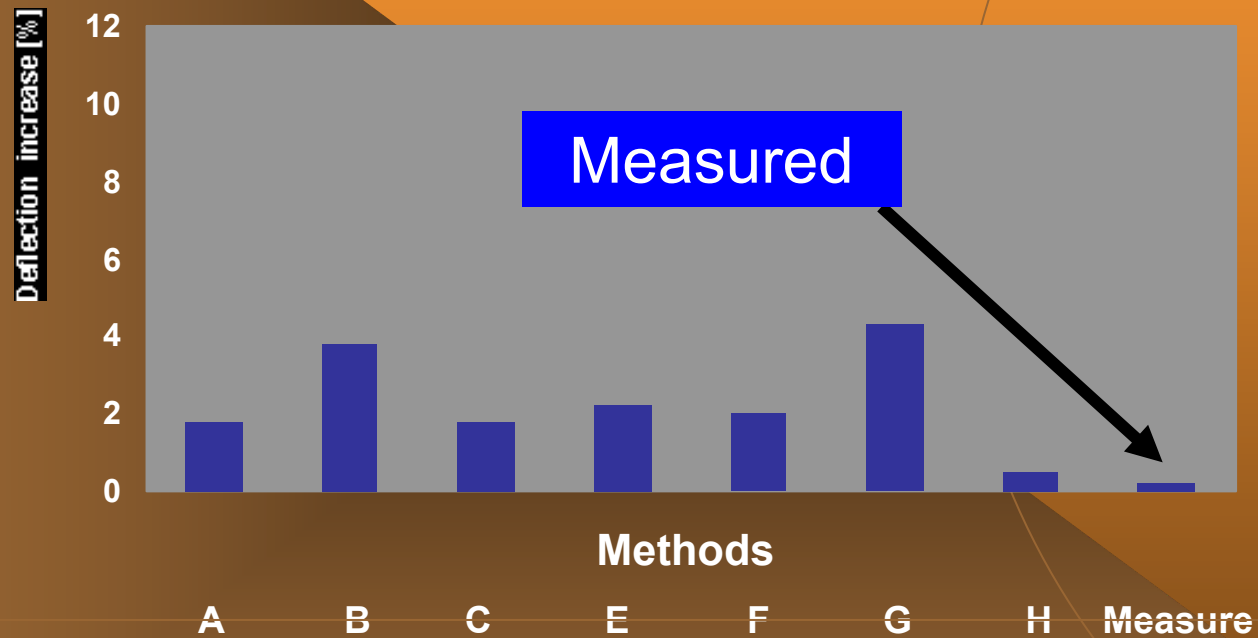
Granular soil, poor installation



# Calculated and measured deflections

Granular soil, poor installation with and without traffic load

Effect of traffic





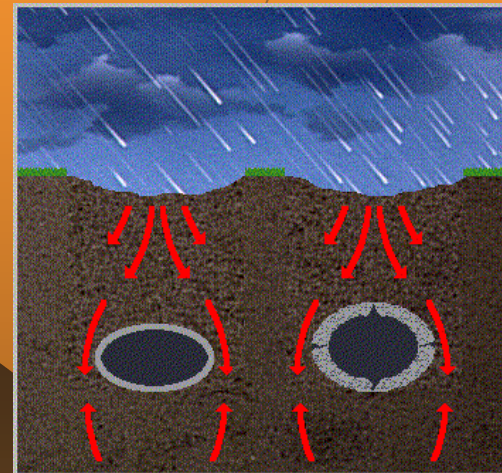
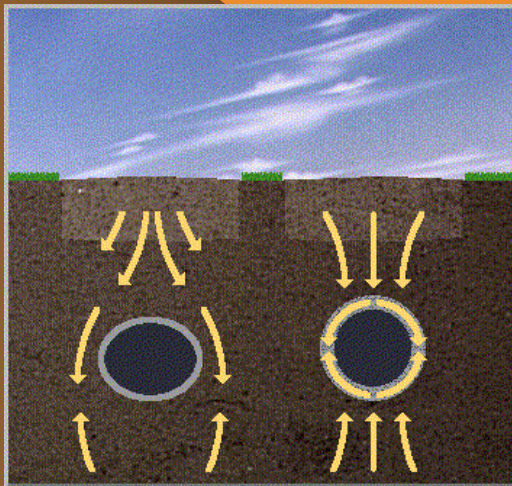
# Summary of the main results

- ◆ Good understanding of soil-pipe interaction.
- ◆ 20 well documented data sets on the different installations.
- ◆ Simplified approach with a new design-tool applicable to the majority of pipe installations.
- ◆ More confidence in plastics pipe performance even under poor installation conditions.



# The pipe soil interaction

Ring deflection of flexible pipes is controlled by the settlement of the soil. After settlement, traffic and other loads do not affect pipe deflection.



**Deflection is safety!**

When pipes are relatively more rigid than the soil, the traffic and other loads have to be resisted by the pipe.

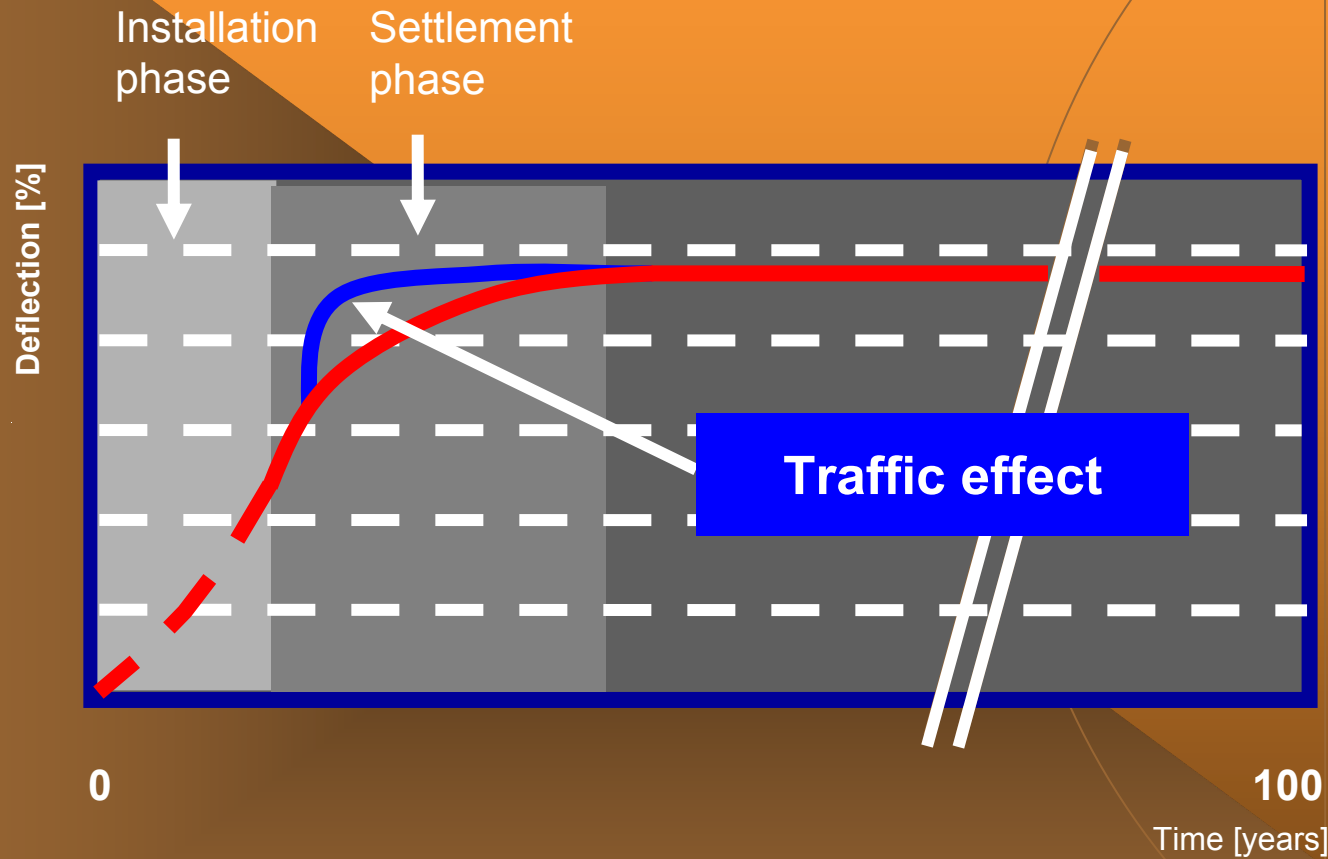


# Facts about deflection

- ◆ Depth of cover is not relevant.
- ◆ Traffic load has no significant effect.
- ◆ Deflection and it's variation depends more on the installation quality than on the pipe stiffness.



# Facts about deflection



# Facts about deflection

- ◆ Recommended max. values :

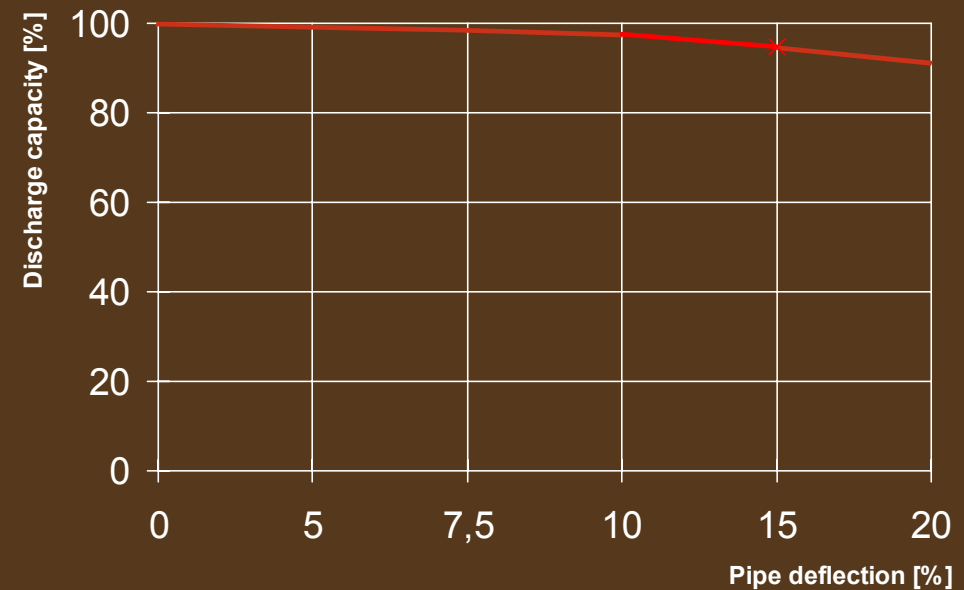
8% initial, 12.5 % final.

(ISO TR 7073)

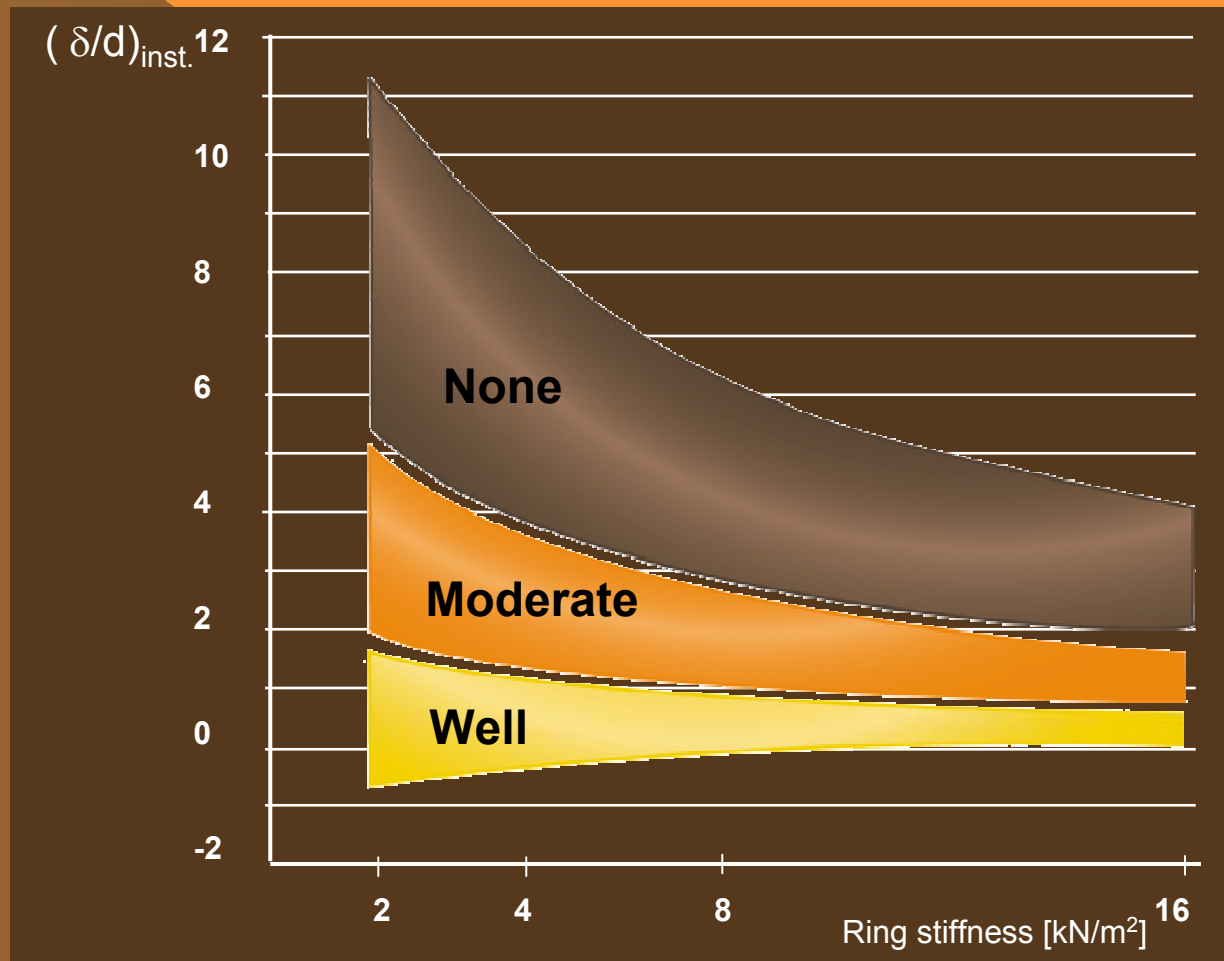
- Pipes deflected up to 10 % - only 2.5 % reduction in discharge capacity.

Deflection is NO issue!

Discharge capacity as a function of pipe deflection



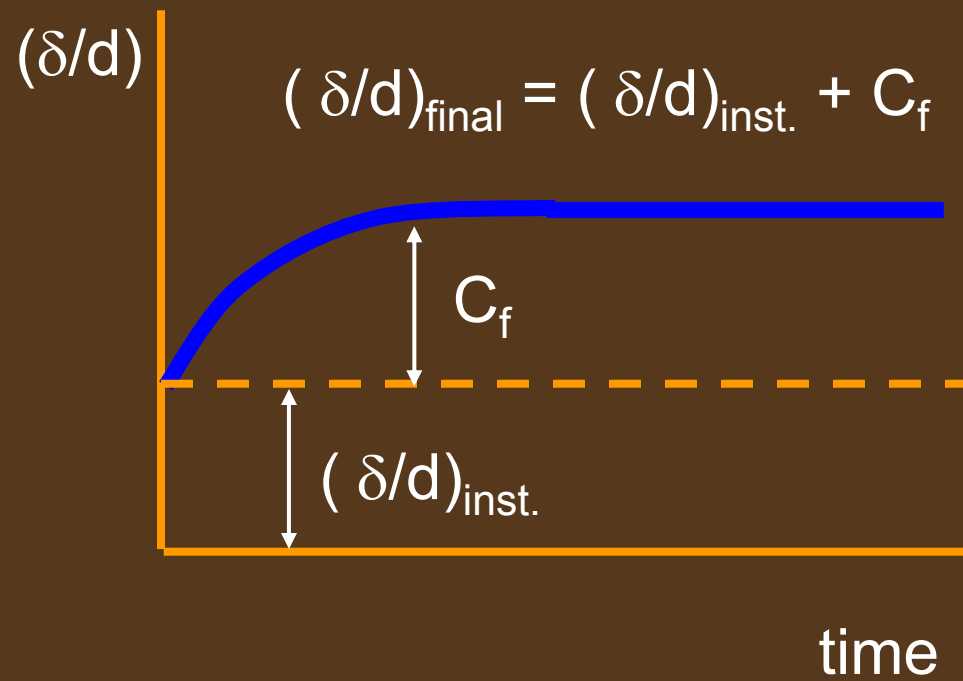
# Pipe deflection after installation



The average deflections immediately after installation are represented by the lower boundary of each area, and the maximum values by the upper boundaries.

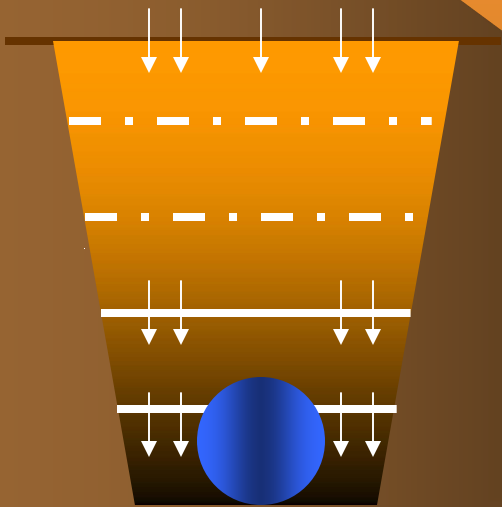


# FJA



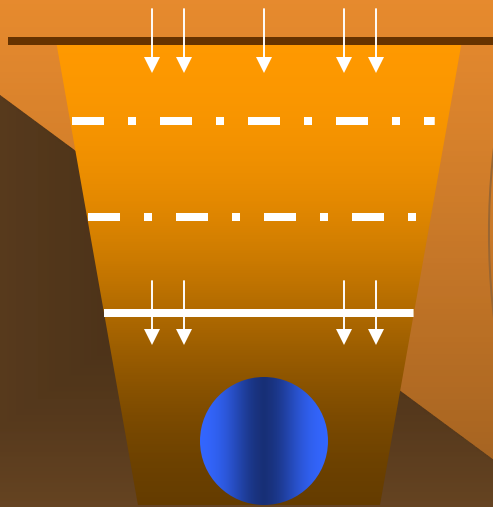
# Installation practices used in the project

$C_f = 1.0$



Well

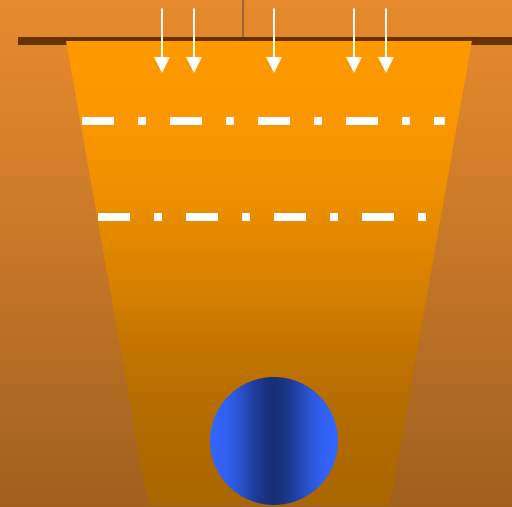
$C_f = 2.0$



Moderate

$C_f$  granular = 3.0

$C_f$  cohesive = 4.0



None





# The paradox

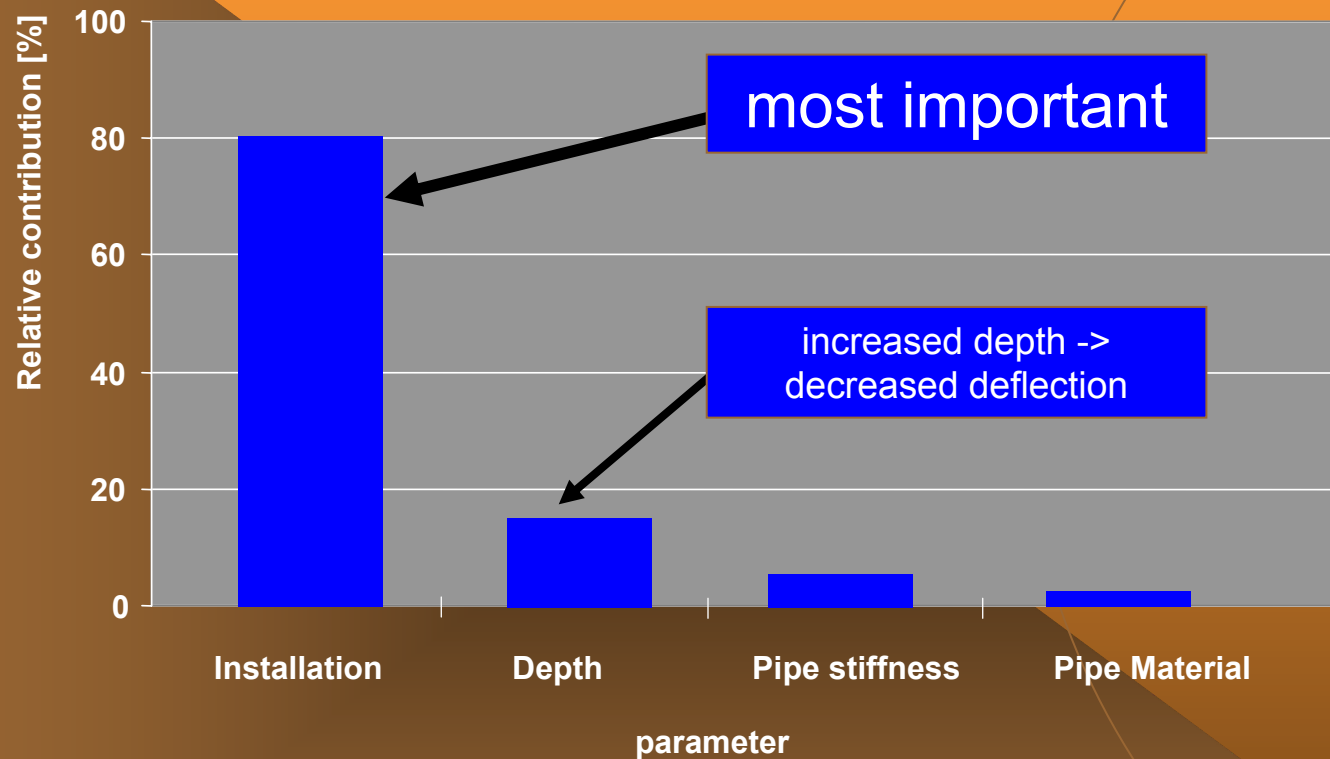
“Sophisticated design methods rely on the quality of the input parameters and that the installation is strict according to the prescriptions.

In such cases a “Well” type of installation is obtained, resulting in very low deflections, and hence design is not important in such cases.

When the quality of the input values is less good, as when installations are becoming more difficult and hence limit state conditions are more likely to occur, sophisticated design methods are no longer appropriate”.



# Effect of parameters on deflection



# Conclusions

- ◆ Depth and traffic load have no effect on the final deflection.
- ◆ For “Well” to “None” type of installation:
  - ◆ pipe stiffness not important
  - ◆ creep ratio / material not important
  - ◆ deflections stay very low
  - ◆ limit state conditions are not likely to occur

Key property : Strainability !



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# Impressions

